Grit Wall analysis

Input data

Material of blocks - filling

Numbei	Name	∕⁄ [kN/m ³]	φ [°]	c [kPa]
1	Rubble Masonry	24.00	45.00	0.00

Material of blocks - mesh

	Number		Strength	Spacing of	Bear.cap.
		Name	overh.	vert. meshes	of front joint
			R _t [kN/m]	b [m]	R _s [kN/m]
	1	Rubble Masonry	0.00	35.00	0.00

Geometry of structure

Number	Width	Height	Offset	Material
vuilibei	b [m]	h [m]	a [m]	Materiai
14	0.30	0.82	0.00	Rubble Masonry
13	0.30	1.20	0.00	Rubble Masonry
12	0.30	1.20	0.00	Rubble Masonry
11	0.30	1.20	0.00	Rubble Masonry
10	0.30	1.20	0.00	Rubble Masonry
9	0.30	1.20	0.00	Rubble Masonry
8	0.30	1.20	0.00	Rubble Masonry
7	0.30	1.20	0.00	Rubble Masonry
6	0.30	1.20	0.10	Rubble Masonry
5	0.40	1.20	0.20	Rubble Masonry
4	0.60	1.20	0.20	Rubble Masonry
3	0.80	1.20	0.20	Rubble Masonry
2	1.00	1.20	1.00	Rubble Masonry
1	2.00	1.20	-	Rubble Masonry

Gabion slope = 45.00 ° Overall height = 10.41 m Overall wall volume = 8.89 m³/m

Soil parameters

Soil Sandy

Unit weight: $\gamma = 19.00 \text{ kN/m}^3$

 $\begin{array}{lll} \text{Stress-state:} & \text{effective} \\ \text{Angle of internal friction:} & \phi_{ef} = 30.00 \, ^{\circ} \\ \text{Cohesion of soil:} & c_{ef} = 0.00 \, \text{kPa} \\ \text{Angle of friction struc.-soil:} & \delta = 10.00 \, ^{\circ} \\ \text{Soil:} & \text{cohesionless} \end{array}$

Saturated unit weight : $\gamma_{sat} = 20.50 \text{ kN/m}^3$

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Geological profile and assigned soils

Number	Layer [m]	Assigned soil
1	15.00	Soil Sandy

Terrain profile

Terrain behind the structure is flat.

Input surface surcharges

Numbor	Surcharge		Action	Mag.1	Mag.2	Ord.x	Length	Depth
Number	new	change	ACTION	[kN/m ²]	[kN/m ²]	x [m]	l [m]	z [m]
1	YES		permanent	5.00				on terrain

Resistance on front face of the structure

Resistance on front face of the structure: passive

Soil on front face of the structure - Soil Sandy

Angle of friction struc.-soil $\delta = 0.00$ ° Soil thickness in front of structure h = 1.10 m

Terrain in front of structure is flat.

Global settings

Active earth pressure calculation - Coulomb Passive earth pressure calculation - Coulomb

Settings of the stage of construction

Analysis carried out according to classical theory (safety factor)

Safety factor for slip = 1.50Safety factor for overturning = 2.00Factor of safety for bearing capacity = 1.00Safety factor for net stress = 1.00

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Verification No. 1

Forces acting on construction

Name	F _{hor}	App.Pt.	F _{vert}	App.Pt.	Design	
	[kN/m]	Z [m]	[kN/m]	X [m]	coefficient	
Weight - wall	0.00	-2.67	213.26	4.83	1.000	
FF resistance	-17.51	-0.37	17.51	2.06	1.000	
Active pressure	56.35	-2.46	-39.46	5.28	1.000	
Surch.1 - surface	2.55	-4.39	-1.79	7.22	1.000	

Verification of complete wall

Check for overturning stability

Resisting moment $M_{res} = 843.79 \text{ kNm/m}$ Overturning moment $M_{ovr} = 143.20 \text{ kNm/m}$

Safety factor = 5.89 > 2.00

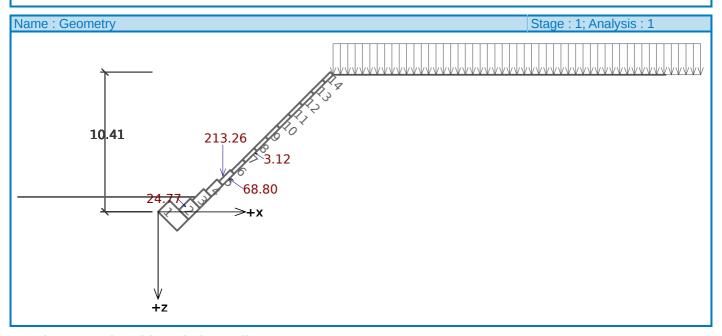
Wall for overturning is SATISFACTORY

Forces acting at the centre of footing bottom

Overall moment M = -537.30 kNm/mNormal force N = 163.29 kN/mShear force Q = -86.19 kN/m

Overall check - WALL is SATISFACTORY

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Bearing capacity of foundation soil

Forces acting at the centre of the footing bottom

Number	Moment	Norm. force	Shear Force	Eccentricity	Stress	
	[kNm/m]	[kN/m]	[kN/m]	[m]	[kPa]	
1	-537.30	163.29	-86.19	0.00	81.64	

Bearing capacity of foundation soil check

Eccentricity verification

Max. eccentricity of normal force e=0.0 mmMaximum allowable eccentricity $e_{alw}=660.0 \text{ mm}$

Eccentricity of the normal force is SATISFACTORY

Footing bottom bearing capacity verification

Max. stress at footing bottom σ = 81.64 kPa Bearing capacity of foundation soil R_d = 180.00 kPa

Safety factor = 2.20 > 1.00

Bearing capacity of foundation soil is SATISFACTORY

Overall verification - bearing capacity of found. soil is SATISFACTORY

Slope stability analysis

Results (Stage of construction 1)

Analysis 1

Circular slip surface

Slip surface parameters							
Contor	x =	-10.21	[m]	Angles :	α ₁ =	-35.28	[°]
Center :	z =	2.27	[m]		α ₂ =	80.61	[°]
Radius :	R =	13.94	[m]				
The slip surface after optimization.							

Slope stability verification (Bishop)

Sum of active forces : $F_a = 675.03 \text{ kN/m}$ Sum of passive forces : $F_p = 1028.21 \text{ kN/m}$ Sliding moment : $M_a = 9404.34 \text{ kNm/m}$ Resisting moment : $M_p = 14324.83 \text{ kNm/m}$

Factor of safety = 1.52 > 1.50 Slope stability ACCEPTABLE