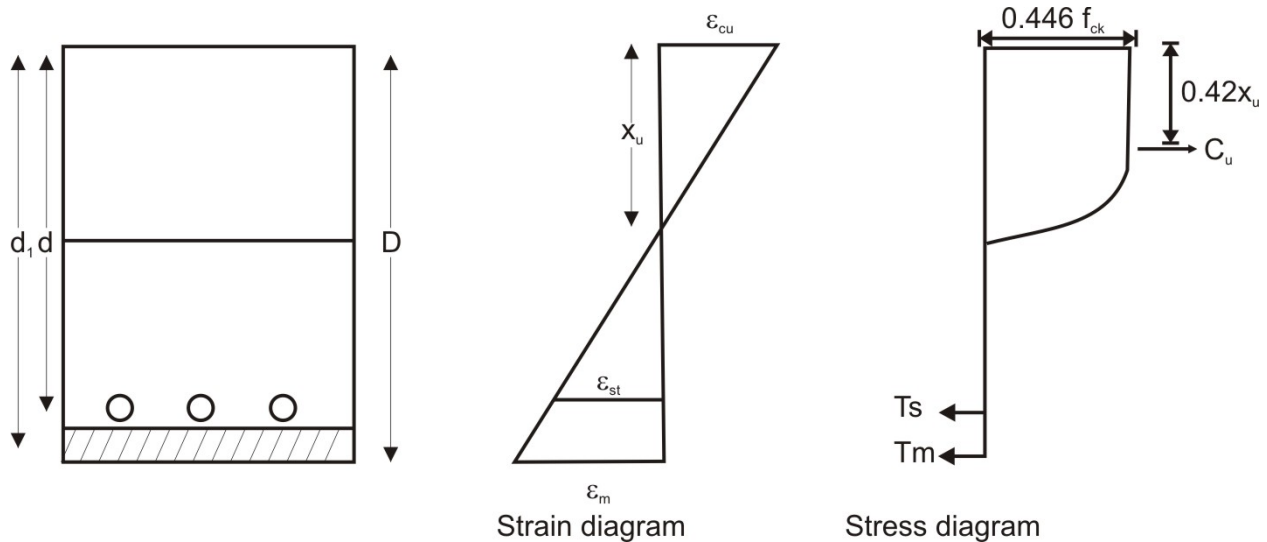


Annexure - 1 Design Stress Block Parameters for Composite Slab



The general expressions for C_u , T_s , T_m can be obtained by adopting partial safety factors for concrete, steel and retrofitting material at the bottom respectively.

As per IS 456:2000

$$\frac{x_u}{d_1} = \frac{\epsilon_{cu}}{\epsilon_{cu} + \epsilon_m} = \frac{0.0035}{0.0035 + \epsilon_m}$$

For $\epsilon_{cu} = 0.0035$

Material	GFRP	MS Plate	Ferrocement
x_u	$0.102d_1$	$0.62d_1$	$0.804d_1$

Where,

ϵ_{cu} = Strain in concrete

ϵ_{st} = Strain in reinforcement bar

ϵ_m = Strain in retrofitted material at the bottom

d = Depth of slab from top up to centre of reinforcement

d_1 = Depth of retrofitted slab from top up to centre of retrofitted material

x_u = Depth of neutral axis from top

C_u = compressive force of concrete

T_s = Tensile force of reinforced bars

T_m = tensile force of retrofitted material

GFRP

Thickness of GFRP layer = 0.8 mm

Here $d = 56$ mm

$d_1 = 75.4$ mm

As per Bhunga and Arora (2008)

Flexural moment of resistance

$$M_u = 0.87 f_y A_{st} (d - 0.42 x_u) + 0.765 A_f E_f \varepsilon_{fe} (d_1 - 0.42 x_u)$$

Assume that the above equation is of the form $M_u = A + B$

Now

$$M_u = 0.87 \times 415 \times 188.4 \times 56 [1 - 0.42 \times 0.137] + 0.765 (0.8 \times 1000) \times 65 \times 10^3 \times 0.004 (75.4 - 0.42 \times 0.466)$$

$$= 2807396.614 + 9658106.64$$

$$M_u = 12.46 \text{ kNm}$$

Now

$$\frac{M_u - A}{A} = \frac{B}{A}$$

$$\frac{M_u - A}{A} = \text{Damage Index}$$

The expression after solving gives a relation between A_f / A_{st} and Damage Index as explained in Fig. 5.46 in *Chapter 5 on 'Results and Discussion'*.

Here

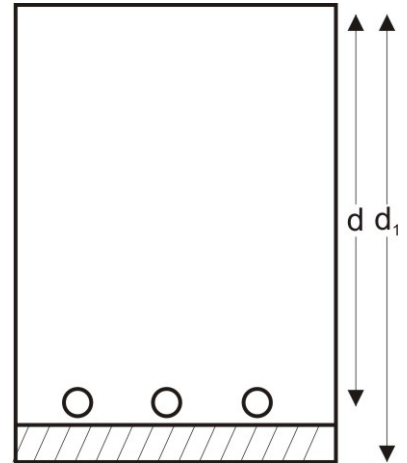
f_y = grade of steel

A_f = cross-section area of FRP

E_f = modulus of elasticity of FRP

A_{st} = area of tensile reinforcement per meter run

ε_{fe} = Effective strain in GFRP



Ferrocement

Thickness of Ferrocement = 25 mm

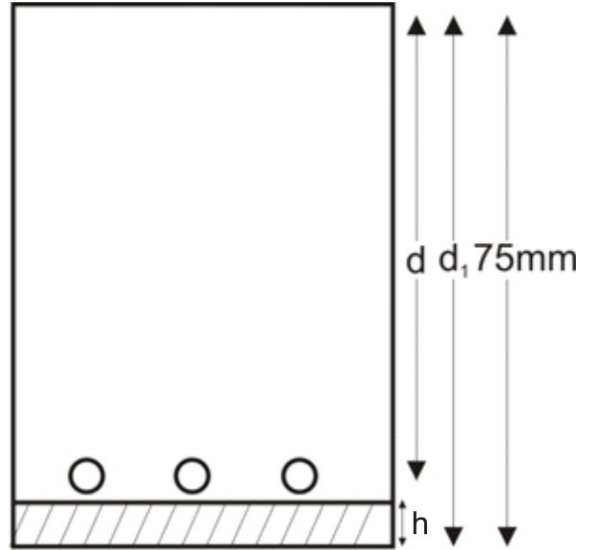
Diameter of bars = 8 mm

$d = 56 \text{ mm}$

$d_1 = 87.5 \text{ mm}$

Overall depth = $75 + 25 = 100 \text{ mm}$

$h = 25 \text{ mm}$



As per Harvinder Singh (2001)

$$M_u = 0.36 f_{ck} b d^2 \left[\frac{x_u}{d} \right] \left[1 - 0.42 \frac{x_u}{D} \right] + f_{ck} b d^2 \left[0.87 \sigma_m k \frac{h}{4 \sigma_{ck} d} \left(\frac{d_1}{d} - 1 \right) \right]$$

Assume that the above equation is of the form $M_u = A + B$

Here $k = \text{constant} = S_F \phi$

$$S_F = \text{specific surface factor} = \frac{s \sigma_y}{\sigma_m}$$

$$s = \text{specific surface area} = \frac{n \pi \phi}{B h}$$

$n = \text{no. of bars per meter width}$

$\phi = \text{diameter of wire mesh}$

$\sigma_y = \text{tensile strength of wire mesh}$

$\sigma_m = \text{tensile strength of cement mortar}$

$b = \text{width of the slab}$

$h = \text{thickness of ferrocement layer}$

$\sigma_{ck} = \text{characteristic strength of concrete}$

$$s = \frac{15 \times 3.14 \times (0.7)}{1000 \times 25} = 0.00131$$

$$S_F = \frac{0.00131 \times 1010}{42} = 0.031$$

$$k = S_F \phi = 0.031 \times 0.7 = 0.022$$

$$\frac{x_u}{d_i} = 0.804 \text{ (for ferrocement)}$$

and

$$\frac{x_u}{D} = 1.25$$

Therefore,

$$M_u = 0.36 \times 20 \times 1000 \times 56^2 \times 1.25 [1 - 0.42 \times 1.25] + 20 \times 1000 \times 56^2 \left[0.87 \times 42 \times \frac{0.039 \times 25}{4 \times 20 \times 56} \left(1.56 - 1 \right) \right]$$

$$= 28224000 [1 - 0.525] + 62720000 \left[\frac{20.09}{4480} (0.56) \right]$$

$$= 13406400 + 157505.6$$

$$= 13563905.6$$

$$= 13.56 \text{ kNm}$$

Now

$$\frac{M_u - A}{A} = \frac{B}{A}$$

$$\frac{M_u - A}{A} = \text{Damage Index}$$

The expression after solving gives a relation between $(\sigma_m kh)$ and Damage Index as explained in Fig. 5.47 in *Chapter 5 on 'Results and Discussion'*.

MS Plate

Thickness of MS plate = 2 mm

Here $d = 56$ mm

$d_1 = 76$ mm

$y_1 =$ Distance of c.g from bottom = 29.54 mm

If $I =$ Moment of Inertia

$Z =$ Section modulus

Flexural moment of Resistance

$$M_u = \sigma Z = \sigma I / y_1 = 2.91 \times 7.61 \times 10^6$$

$$= 16.6 \text{ kNm}$$

For S1 Slab

$$M_u = A + B$$

$$\frac{M_u - A}{A} = \frac{B}{A}$$

$$\frac{M_u - A}{A} = \text{Damage Index}$$

Where $A =$ Moment of control slab

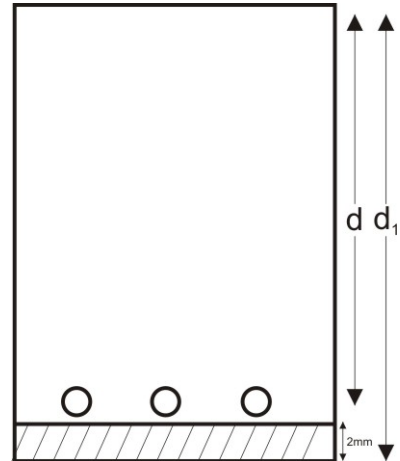
$B =$ Moment of retrofitted portion

$$= 5 + 0.87 \times (2 \times t) \times 250 \times 76 [1 - 0.42 \times 0.62]$$

$$= 2.44t$$

Similarly for S2 Damage Index = 2.84t

The thickness of mild steel plate can be found out for different values of Damage Index as given in Fig. 5.48 in *Chapter 5* on 'Results and Discussion'.



For Slab S1

Property Material	M.O. R (M_u) (kNm)	Control Slab (M_u) (kNm)	Difference in flexural strength (M_u) (kNm)
GFRP	12.46	5	7.46
MS plate	13.56	5	8.56
FC	16.6	5	11.6

For Slab S2

Property Material	M.O. R (M_u) (kNm)	Control Slab (M_u) (kNm)	Difference in flexural strength (M_u) (kNm)
GFRP	12.46	4.37	8.09
MS	13.56	4.37	9.19
FC	16.6	4.37	12.23